

## Soil Response to Earthquake Excitation—SRES

When the thickness, unit volume weight, shear modulus, and damping coefficient of each layer in a horizontally stratified ground are given and the input acceleration time history at the top of any reference layer is specified, the program SRES (Soil Response to Earthquake Excitation) is a subroutine subprogram that calculates the response acceleration time history at the top of any objective layer.

### SRES (Soil Response to Earthquake Excitation )

#### 【Purpose】

When an input acceleration time history at the top of the specified reference layer in a horizontally stratified ground, the acceleration response time history and its maximum value at the top of the specified objective layer are calculated using the frequency response function.

#### 【Usage】

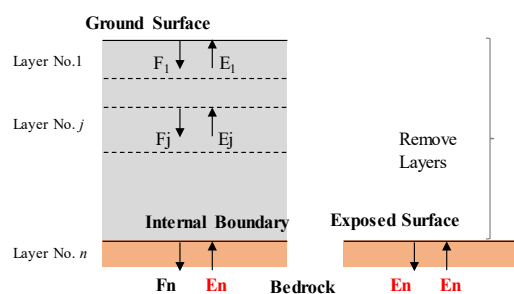
( 1 ) How to connect

CALL SRES (L, TH, UW, G, ALPHA, BETA, ND1, LOBJ, LREF, DT, NN, DDY, ACC, ND2,  
ACMAX, IND)

Argument	Type	Parameter in calling program	Return Parameter
L	I	Total number of layers including bottomstratum (L.LE. 60)	Unchanged
TH	R 1-D array ( ND1 )	Thickness of each layer (unit : m)	Unchanged
UW	R 1-D array ( ND1 )	Unit volume weight of each layer (unit : tf/m <sup>3</sup> )	Unchanged
G	R 1-D array ( ND1 )	Elastic shear modulus of each layer (unit : tf/m <sup>2</sup> )	Unchanged
ALPHA	R 1-D array ( ND1 )	Scatter damping coefficient of each layer (unit : 1/sec)	Unchanged
BETA	R 1-D array ( ND1 )	Material damping coefficient of each layer (unit : nondimension )	Unchanged

ND1	I	Dimension size of TH, UW, G, ALPHA, and BETA in calling program	Unchanged
LOBJ	I	Number of objective layer	Unchanged
LREF	I	Number of reference layer	Unchanged
DT	R	Time increment of acceleration time history (unit : Sec)	Unchanged
NN	I	Total number of acceleration data (NN.LE. 8192)	Unchanged
DDY	R 1-D array ( ND2 )	Acceleration time history at surface of reference layer (unit : Gal)	Unchanged
ACC	R 1-D array ( ND2 )	No need to input here	Acceleration time history at surface of objective layer (unit : Gal)
ND2	I	Dimension size of DDY and ACC in calling program (ND2.GE. NN)	Unchanged
ACMAX	R	No need to input here	Maximum acceleration at surface of objective layer (ACC) (unit : Gal)
IND	I	See the following Table	Unchanged

IND		Reference Layer	
		Internal	Exposed
Objective Layer	Internal	11	12
	Exposed	21	-



(2) Necessary subroutines and function subprograms

FESP FAST

(3) Remarks

- i) Layer thickness of the bottom stratum  $TH(L)$  is not necessary.
- ii) The ground surface is always considered the internal boundary surface.
- iii) If  $IND = 21$ , the objective layer must be located deeper than the reference layer.
- iv) The case that both the reference and objective layers are exposed boundaries is not permitted.

### 【Calculation Method】

The calculation is carried out in the following steps.

- i) The acceleration time history  $\xi_s(t)$  of the input seismic motion given to the reference layer  $s$  is Fourier transformed by the Fast Fourier Transform program **FAST**, after adding subsequent zeros if

necessary, and the harmonic vibration component  $F_s(\omega)$  is obtained.

- ii) The frequency response function  $Z_{r/s}(\omega)$  of the objective layer  $r$  to the reference layer  $s$  is calculated by the program **FESP** using the given thickness  $H$ , unit volume weight  $\gamma$ , shear modulus  $G$ , and damping factors  $\alpha$  and  $\beta$  of each layer.
- iii) Multiply  $Z_{r/s}(\omega)$  by  $F_s(\omega)$  to obtain the harmonic vibration component of the response,  $F_r(\omega)$ .
- iv) The time history of the response acceleration  $\ddot{\xi}_r(t)$  in the objective layer  $r$  is obtained by performing an inverse Fourier transform of  $F_r(\omega)$  using the program **FAST** again.
- v) Find the maximum value of the response acceleration time history  $\ddot{\xi}_r(t)$ .

Both the reference and the objective layers can be treated as internal or exposed boundary surfaces. The combination of the two layers can be specified by the value of the argument  $IND$ , but neither the reference layer nor the objective layer can be exposed boundary surface. And if the objective layer is exposed and the reference layer is internal ( $IND = 21$ ), there are two things to note. One is that the objective layer must be located deeper than the reference layer. The other is that this program calls subroutine **FESP** to calculate the frequency response function, but **FESP** cannot handle the case where the objective layer is exposed. So, in this case, the frequency response function is calculated assuming that the objective layer is internal, and the reference layer is exposed ( $IND2 = 1$  in **FESP**), and the inverse of the obtained frequency response function is computed.

This program assumes that the shear modulus and damping coefficient of each layer are constant, i.e., not strain-dependent. Therefore, this program can only be applied in the case of small seismic motions where the maximum shear strain of each layer is almost within 0.01% for all layers. The method of calculating the maximum shear strain is shown in [Example 2] of the program **FESP**.

#### 【Program List】

C	*****	SRES	1
C	SUBROUTINE FOR SOIL RESPONSE TO EARTHQUAKE MOTION	SRES	2
C	*****	SRES	3
C		SRES	4
C	CODED BY Y. OHSAKI	SRES	5
C		SRES	6
C	PURPOSE	SRES	7
C	TO COMPUTE, BY MEANS OF FREQUENCY RESPONSE FUNCTION, THE RES-	SRES	8
C	PONSE ACCELERATION TIME-HISTORY AT THE TOP OF THE SPECIFIED	SRES	9
C	OBJECTIVE LAYER IN A HORIZONTALLY LAYERED SOIL DEPOSIT, WHEN	SRES	10
C	AN INPUT ACCELERATION TIME-HISTORY IS GIVEN AT THE TOP OF THE	SRES	11
C	SPECIFIED REFERENCE LAYER.	SRES	12
C		SRES	13
C	USAGE	SRES	14
C	CALL SRES(L, TH, UW, G, ALPHA, BETA, ND1, LOBJ, LREF, DT, NN, DDY, ACC,	SRES	15
C	ND2, ACMAX, IND)	SRES	16
C		SRES	17
C	DESCRIPTION OF ARGUMENTS	SRES	18
C	L - TOTAL NUMBER OF LAYERS, INCLUDING THE BASE LAYER	SRES	19
C	L. LE. 60	SRES	20
C	TH(ND1) - THICKNESS IN METERS	SRES	21
C	UW(ND1) - UNIT WEIGHT IN TONS/CU. METER	SRES	22
C	G(ND1) - SHEAR MODULUS IN TONS/SQ. METER	SRES	23
C	ALPHA(ND1) - SCATTER DAMPING COEFFICIENT IN 1/SEC	SRES	24

C	BETA(ND1)	- MATERIAL DAMPING COEFFICIENT IN DECIMAL FRACTION	SRES	25
C	ND1	- DIMENSION OF TH, UW, G, ALPHA BETA IN CALLING PROGRAM	SRES	26
C	LOBJ	- NUMBER OF OBJECTIVE LAYER	SRES	27
C	LREF	- NUMBER OF REFERENCE LAYER	SRES	28
C	DT	- TIME INCREMENT IN SEC	SRES	29
C	NN	- TOTAL NUMBER OF DATA IN THE TIME-HISTORIES	SRES	30
C		NN. LE. 8192	SRES	31
C	DDY(ND2)	- REFERENCE ACCELERATION TIME-HISTORY IN GALS	SRES	32
C	ACC(ND2)	- OBJECTIVE ACCELERATION TIME-HISTORY IN GALS	SRES	33
C	ND2	- DIMENSION OF DDY, ACC IN CALLING PROGRAM	SRES	34
C	ACMAX	- MAX. ACCELERATION OF ACC IN GALS	SRES	35
C	IND	- SEE THE FOLLOWING TABLE	SRES	36
C		-----	SRES	37
C	I	I REFERENCE LAYER I	SRES	38
C	I	IND I-----I	SRES	39
C	I	I INTERNAL I EXPOSED I	SRES	40
C	I	I-----I-----I	SRES	41
C	I	I INTERNAL I 11 I 12 I	SRES	42
C	I	I OBJECTIVE LAYER I-----I-----I	SRES	43
C	I	I EXPOSED I 21 I NO I	SRES	44
C		-----	SRES	45
C			SRES	46
C	REMARKS		SRES	47
C	(1)	THICKNESS OF BASE LAYER TH(L) IS NOT REQUIRED	SRES	48
C	(2)	THE GROUND SURFACE IS ALWAYS REGARDED AS INTERNAL	SRES	49
C	(3)	WHEN IND=21, THE OBJECTIVE LAYER MUST BE LOCATED DEEPER	SRES	50
C		THAN THE REFERENCE LAYER	SRES	51
C	(4)	THE CASE THAT BOTH REFERENCE AND OBJECTIVE LAYERS ARE	SRES	52
C		EXPOSED IS NOT PERMITTED	SRES	53
C			SRES	54
C	SUBROUTINES AND FUNCTION SUBPROGRAMS REQUIRED		SRES	55
C	FESP FAST		SRES	56
C			SRES	57
C	SUBROUTINE SRES(L, TH, UW, G, ALPHA, BETA, ND1, LOBJ, LREF, DT, NN, DDY,		SRES	58
C	* ACC, ND2, ACMAX, IND)		SRES	59
C			SRES	60
C	DIMENSION TH(ND1), UW(ND1), G(ND1), ALPHA(ND1), BETA(ND1), DDY(ND2),		SRES	61
C	* ACC(ND2)		SRES	62
C	COMPLEX H(4097), C(8192)		SRES	63
C			SRES	64
C	FOURIER TRANSFORM OF REFERENCE TIME-HISTORY		SRES	65
C			SRES	66
C	NT=2		SRES	67
C	110 IF(NT. GE. NN) GO TO 120		SRES	68
C	NT=NT*2		SRES	69
C	GO TO 110		SRES	70
C	120 NFOLD=NT/2+1		SRES	71
C	DO 130 M=1, NN		SRES	72
C	C(M)=CMPLX(DDY(M)/REAL(NT), 0.)		SRES	73
C	130 CONTINUE		SRES	74
C	IF(NN. EQ. NT) GO TO 150		SRES	75
C	DO 140 M=NN+1, NT		SRES	76
C	C(M)=(0., 0.)		SRES	77

```

140 CONTINUE SRES 78
150 CALL FAST(NT, C, 8192, -1) SRES 79
C SRES 80
C FREQUENCY RESPONSE FUNCTION SRES 81
C SRES 82
DF=1. /REAL(NT) /DT SRES 83
IF(IND. EQ. 21) GO TO 160 SRES 84
CALL FESP(L, TH, UW, G, ALPHA, BETA, ND1, LOBJ, LREF, DF, NFOLD, H, 4097, 0,
* IND-11) SRES 86
GO TO 180 SRES 87
160 CALL FESP(L, TH, UW, G, ALPHA, BETA, ND1, LREF, LOBJ, DF, NFOLD, H, 4097, 0, 1) SRES 88
DO 170 K=1, NFOLD SRES 89
H(K)=(1., 0.) /H(K) SRES 90
170 CONTINUE SRES 91
C SRES 92
C OBJECTIVE TIME-HISTORY SRES 93
C SRES 94
180 C(1)=C(1)*H(1) SRES 95
DO 190 K=2, NFOLD-1 SRES 96
C(K)=C(K)*H(K) SRES 97
C(NT-K+2)=CONJG(C(K)) SRES 98
190 CONTINUE SRES 99
C(NFOLD)=C(NFOLD)*H(NFOLD) SRES 100
CALL FAST(NT, C, 8192, +1) SRES 101
ACMAX=0. SRES 102
DO 200 M=1, NN SRES 103
ACC(M)=REAL(C(M)) SRES 104
ACMAX=AMAX1(ACMAX, ABS(ACC(M))) SRES 105
200 CONTINUE SRES 106
RETURN SRES 107
END SRES 108

```

### 【Example1】

For the ground shown in the table below, read the acceleration time history observed at the top of the base stratum at a depth of 10.90 m from the file EQ.03. Give it as input and compute the response acceleration time history at the ground surface.

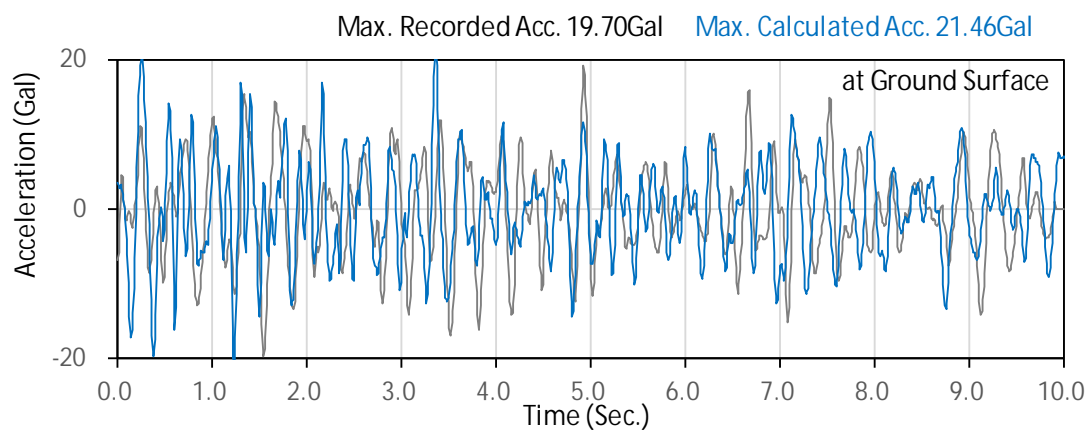
Layer No.	Upper depth (m)	Layer thickness (m)	Unit volume weight (tf/m <sup>3</sup> )	Elastic shear modulus (tf/m <sup>2</sup> )
1	0.00	3.80	1.50	1200
2	3.80	3.20	1.67	2900
3	7.00	3.90	1.85	5700
4	10.90	-	1.95	50000

```

C
  DIMENSION TH(4), UW(4), G(4), ALPHA(4), BETA(4), DDY(1000),
*          ACC(1000)
  DATA    L/4/, TH/3.8, 3.2, 3.9, 0.0/, UW/1.50, 1.67, 1.85, 1.95/,
*          G/1200., 2900., 5700., 50000/, ALPHA/4*2.0/, BETA/4*0.02/,
*          LOBJ/1/, LREF/4/, IND/11/
C
  READ(5, 501) DT, NN, (DDY(M), M=1, NN)
  CALL SRES(L, TH, UW, G, ALPHA, BETA, 4, LOBJ, LREF, DT, NN, DDY, ACC, 1000,
*          ACMAX, IND)
  STOP
C
501 FORMAT(T51, F10.0, I10/(8F10.0) )
      END

```

Output: Calculated results are stored in the array *ACC* and plotted as the blue line in the following figure. The acceleration time history in the file EQ.02 (recorded at the ground surface at the same time) is also shown in this figure in gray line for reference.



Note: The maximum response acceleration is different from that in the Japanese version manual. This is because the value of attenuation *ALPA* was set to 2.0 in the English version, the same as in Dr. Ohsaki's book, while it was 1.8 in the Japanese version. This is also true for the following example.

### 【Example2】

Using the same ground model as in Example 1, read the acceleration time history on the ground surface from file EQ.02, and remove the surface soil up to a depth of 10.90 m to expose the upper plane of the base stratum and calculate the acceleration time history at this plane.

```

C
  DIMENSION TH(4), UW(4), G(4), ALPHA(4), BETA(4), DDY(1000),
*          ACC(1000)
  DATA    L/4/, TH/3.8, 3.2, 3.9, 0.0/, UW/1.50, 1.67, 1.85, 1.95/,
*          G/1200., 2900., 5700., 50000/, ALPHA/4*2.0/, BETA/4*0.02/,
*          LOBJ/4/, LREF/1/, IND/21/

C
  READ(5, 501) DT, NN, DDYMAX, (DDY(M), M=1, NN)
  WRITE(6, 601) DDYMAX
  CALL SRES(L, TH, UW, G, ALPHA, BETA, 4, LOBJ, LREF, DT, NN, DDY, ACC, 1000,
*          ACMAX, IND)
  WRITE(6, 602) ACMAX
  STOP

C
501 FORMAT(T51, F10.0, I10, F10.0/(8F10.0) )
601 FORMAT(' MAX ACCELERATION' /T3, ' GROUND SURFACE', F10.2, TR1, ' (GAL)')
602 FORMAT(T3, ' EXPOSED ROCK ', F10.2, TR1, ' (GAL)')
  END

```

Output: Calculated results are stored in the array *ACC* and the maximum acceleration is 8.94 Gal as shown below.

```

MAX ACCELERATION
  GROUND SURFACE      19.70 (GAL)
  EXPOSED ROCK        8.94 (GAL)

```